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64-9

ERR-FW-196 15 January 1964



FACE WRINKLING AS A FUNCTION
OF SURFACE WAVINESS

Published and distributed under Contract No. AF33(657)-11214, Air Force Materials Laboratory, Aeronautical Systems Division, Air Force Systems Command, Wright-Patterson Air Force Base, Ohio.



GENERAL DYNAMICS FORT WORTH



### FACE WRINKLING AS A FUNCTION OF SURFACE WAVINESS

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6 May 1963

### RESEARCH & ENGINEERING DEPARTMENTS

This work was supported under General Dynamics/Fort Worthsponsored research program REA 14-61-597

GENERAL DYNAMICS | FORT WORTH

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#### NOTATION

A<sub>o</sub> - Amplitude of initial waviness contained within a critical half-wave length

At - Amplitude of the total initial waviness

b - Thickness of core

Ec - Young's modulus of core in a direction perpendicular to

Ef - Young's modulus for faces

G - Shear modulus of core

K - Modulus of foundation

Length of initial waviness

 $l_1$  - Critical half-wave length of wrinkling

 $\chi_2$  - Length of changing waviness

t - Thickness of face

Ncr - Critical load per inch

F<sub>c</sub> - Ultimate tensile or compressive strength of core or bond

 $F_8$  - Ultimate shear strength of core

I - Moment of inertia

Ofw - Critical face-wrinkling stress

W - Depth of effective core for one face

w&l - Subscripts indicating transverse or longitudinal direction of core

0 - Numerator of critical wrinkling equation

#### SUMMARY

An attempt was made to establish the correlation between surface waviness of honeycomb core panels and the face wrinkling attendant upon the panels when subjected to compressive forces. Tests were conducted with honeycomb sandwich edgewise compression specimens with controlled initial surface waviness. A survey of the literature disclosed that previous investigations furnished an excellent base for advancing the state of the art of this widely used form of construction. Although previous researchers had suggested methods of predicting the effects of surface waviness on skin wrinkling when considering only a small degree of waviness, the current testing indicated that by extrapolation these predictions could be applied to panels having a large degree of waviness. Further investigation must be conducted to obtain an empirical constant that will eliminate the problem of estimating or measuring the initial surface waviness.

#### 1. INTRODUCTION

The structural concept described by the term "sandwich" has been well defined analytically (Ref. 1) in all of its local modes of failure with the exception of the term "face wrinkling". The currently accepted method of analysis outlined by Kuenzi (Ref. 2) does not apply throughout the range of present-day usage.

"Face wrinkling" is a very localized swippling of one face only. The wrinkle may go into or away from the strength of the core and the core-to-face bond.

S. Yusuff (Ref. 3) proposed a solution pased on the "beam on an elastic foundation" theory. The basic mackling equation is shown by Hetenyi (Ref. 4) to be

$$N_{CT} = 2 \sqrt{KEI}. \tag{1}$$

Yusuff relates the parameters of the above equation in terms of the sandwich components and includes the effect of initial waviness on the failing stress.

In this paper we will discuss the solution proposed by Yusuff and present the results of a recent series of tests designed to explore the effect of surface waviness.

### 2. DISCUSSION

The results of Yusuff's work are presented here for discussion purposes.

if b < 2W (thin core)

$$O_{fw} = \frac{\sqrt{\frac{2}{3}} E_f E_c \frac{t}{b}}{1 + \frac{2 A_o E_c}{F_o b}}; \quad \text{(for core compression or tension failure)}$$

if b > 2W (thick core)

$$O_{fw} = \frac{.96 \quad \sqrt[3]{(E_f E_c, G_e)}}{1 + \frac{E_c A_o}{W F_o}}, \text{(for core compression or tension failure)}$$

$$O_{fW} = \frac{.96 \quad \sqrt[3]{\frac{E_f E_c G_c}{E_f E_c G_c}}}{1 + \frac{A_o G_c}{F_s l_i}} \quad \text{(for shear failure of the core)}$$

The critical half-wave length of wrinkling, as expressed by Hetenyi, is:

$$\mathcal{L}_{1} = \frac{\pi}{\frac{4}{\sqrt{\frac{G E_{c}}{b E_{c} t^{3}}}}}$$
 (6)

In order to apply the above theory, a practical definition of waviness must be established. The writer has found that two distinct forms of surface waviness predominate; however, many others exist. These two are defined as follows:

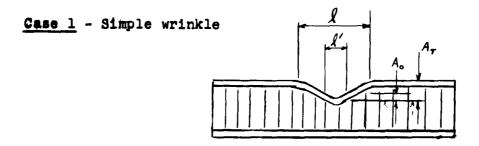


Figure 1 WRINKLE GEOMETRY CASE 1

by inspection

$$A_0 = A_t \frac{\ell_1}{\ell} . \tag{7}$$

Case 2 - Compound wrinkle

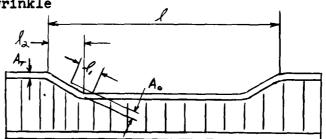


Figure 2 WRINKLE GEOMETRY CASE 2

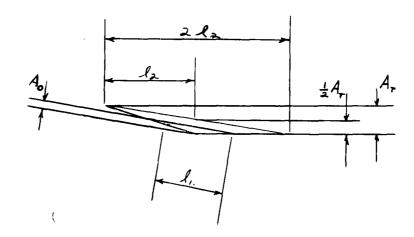


Figure 3 A RELATIONSHIP CASE 2

Using the geometrical relationships shown in Figure 3, the following approximation can be made.

$$\frac{A_0}{\ell_1} = \frac{1/2 A_T}{2 \ell_2} \quad ,$$

then

$$A_0 = \frac{A_T \ell_1}{4 \ell_2} . \tag{8}$$

The significant point brought out by the definition of  $A_0$  for both Case 1 and Case 2 is that the critical amplitude,  $A_0$ , is the amplitude of the surface waviness contained within a critical half-wave length. This point is one of the basic assumptions in Yusuff's derivation.

#### 2.1 Test Specimens

Considerable experience has been gained in testing small 3- by 4-inch edgewise compression specimens of sandwich construction. These specimens have proven particularly valuable in evaluating the critical face-wrinkling stress for a particular type of sandwich. It is necessary to fill the loaded edges of the specimen with a suitable filler to prevent local failure. Hetenyi shows that the critical buckling stress for a semi-infinite column on an elastic foundation is half that of an infinite column because of the lack

1

of face moment continuity. This relationship is applicable to the loaded edge of a sandwich specimen and must be overcome by increasing the foundation stiffness. If the core cell size is relatively large, the sides must be filled also.

Twelve-inch-square sandwich panels were fabricated with the face material and core shown in Figure 4 and Table 1. Metlbond 408 was selected as the adhesive because of its excellent strength and wetting properties. This adhesive flows to the cell wall-face juncture to form a strong fillet without the aid of a glass cloth carrier. Consequently, there is no added stiffness in the faces because of the adhesive. Adhesives with a glass cloth carrier appreciably increase the apparent stiffness of relatively thin faces.

In an effort to obtain a controlled waviness, two wires were placed on the platen side of each panel as shown in Figure 4. wires were .003 inch and .005 inch in diameter.

3	x 8 Beam (1)	3 x 4 Col. (2)
3 x 4 Col. (3)	3 x 4 Col.(4)	
3 x 4 Col. (6)	3 x 4 Col.(7)	3 x 4 Col. (8)
3 x 4 Col. (19)	3 x 4 Col.(10)	3 x 3 Flat (11) Comp.
¥.	003 Wire	.005 Wire

# CORE PROPERTIES (Ref. 5)

NOTES: 1. Numbers in ( ) are specimen numbers

#### TYPE IA 5052 ALUMINUM

 $F_{gw} = 120 \text{ psi}$ 

 $F_{sL} = 210 \text{ psi}$ 

 $F_c = 270 \text{ psi}$ 

 $E_0 = 130,000 \text{ psi}$ 

 $Q_{QW} = 21,500 \text{ psi}$ 

 $G_{c.L} = 31,500 \text{ psi}$ 

- 2. Beam flex test specimens are
- 3- by 8- inches
- 3. Column compression test specimens are 3- by 4- inches
- 4. Flatwise compression specimens are 3- by 3-inches

Figure 4 FACE-WRINKLING PANELS CUTTING PLAN

Table 1
TEST SPECIMENS CONFIGURATION

			CORE		COMPUTED	
PANEL	FACE MATERIAL	GAGE	THICKNESS	8	$\gamma_1$	W
A	7075 TG	.010	0.50	295,000	0.16	0.104
В	7075 TG	.010	05.0	295,000	0.16	0.104
δ	7075 TG	.010	0.20	210,000	0.128	0.104
А	7075 TG	.010	0.20	210,000	0.128	0.104
田	17-7 PH (TH 1050)	.008	0.50	418,000	0.1775	0.116
ርኳ	17-7 PH (TH 1050)	800.	0.50	418,000	0.1775	0.116
<b>U</b>	17-7 PH (TH 1050)	800.	0.25	294,000	0,149	0.116
н	17-7 РН (ТН 1050)	.008	0.25	294,000	0.149	0.116

Thin, high-strength faces combined with minimum stiffness core were selected in an effort to obtain wrinkling failures when the face stresses were still in the elastic range. Core thicknesses were arbitrarily set to evaluate both thick and thin core solutions.

Up to this point in the program, everything proceeded as planned. Because of the nature of the problem, an extremely fragile specimen construction was required. The problems resulting from specimen preparation were greater than estimated; three major discrepancies were found in the completed specimens. First, the faces did not remain parallel when the core was removed from the ends for filling. The ends were generally spread or not filled to a sufficient depth. Second, the sawing operations on the steel-faced specimens damaged the core cells adjacent to the saw cut, necessitating filling of all edges. Third, a large number of specimens were crushed during machine milling of the loading ends.

In spite of the bad specimen preparation, all but three specimens were tested. Prior to testing, the surface waviness was measured on a flat granite table with a supported dial gage. At least three traverses were made on each side and any significant waviness was recorded on the face of the specimen. Both length and depth were noted.

The specimens were all tested at room temperature in a 12,000-pound Baldwin hydraulic test machine at a load rate of 1,200 pounds per minute. No loading jigs were used.

#### 2.2 Test Results

The results of the waviness survey, together with failing load, description, and a computed A<sub>O</sub>, are shown in Tables 2 through 5. A plot of the actual failing stress versus the computed A<sub>O</sub>, together with a curve as computed by the appropriate equation, are shown for each type of panel in Figures 5 through 8. All results are shown except for those cases wherein the failures did not occur at a measured irregularity and were unquestionably caused by improper edge preparation.

THEST RESOLTS - PARKES A SEE

2.10 2.10 2.10	-		1		EUTTO:		LOAD IN		THI	TALLOTE	•
2.10 2.10	;.	7						1	110 110 1		-
0 80	+::		+	Ľ.	17	42	POUNDS	PSI	LOCATION	TYPE	d:
2.70		ı	1	elo. Elo.	0.8	0.33/0.3,	2300	41,400	Bot 3 to	FK	. 0022
23 6	5	ı	1	.002		1	2500	,2,200	End	'	.000
	0	1	1	.0025	0.0	l	3930	71,000	End *	ı	.0008
A-4   2./6	,011/	,	, /00.0	/110.	,	. /1.0	2250	40,000	Botn 3 40	3 <b>8</b> P.	.001
A-, 2.78	.012,007	1.5	0.2:/1.0	400.	·.0		2970	33,400	Both "	¥	.0011B
2.10	.021/	,	//:0	,020,	1	0.40/	1965	3,,400	Botn '	Z	.OJ2CB
A-1 2.70	.020/	ı	0.60/	.024/	1	. /04.0	2010	36,100	Воти но	78. EL	.002+B
A-è 2.73	510./600.	2.25	0.60/0.43	.035/	Corner	1	2130	36,600	Top 3 no	SS	.0012T
4-9 2.78	.002/	٥٠.٥	l	//.00.	0.3 end		2500	46,000	End		.0036B
9 2.79	.002/	0.50	1	0	,	1	3250	58,300	Top & Ao*	Z	.001
1-10 2.78	/900-	ı	0.3//	/‹‹00.	ı	, 705.0	3005	22,100	Both & Ao	£	.00 <b>091</b>
B-2 2.83 1.	,010,		0.0	.020/		0.25/	18,0	32,700	Bot ₹ 40	SS	.0032B
B-3 2.43	.003/	1.0	4- x	0	0		5725	000,30	Top & Ao	7	.0005
B-4 2.43	,01.7.004	1.10	0.63/0.43	, v20/	1	/05.0	1910	33,600	Bot & Ao	36	.0016B
B-, 2.63	.012	ı	6.43/	.000	ı	0.22/	2460	43,600	Both & Ao	¥	.001 B
B-6 2.83	.016/	1	// // // 0	.027/	ı	٠.٥٥/	20ر1	26,800	Bot & Ao	F	.0013B
B-/ 2.43	/210.	1	1.25/	,029/	1	0.24/	1520	32,000	Bot & Ao	SS	.0047B
B-6 2.63	,010,	1	//:0	,018/	,	, , , , ,	2000	36,000	Both & 40	FW	.0014B
B-9 2.83	031/		0.30/	.020/		/::0	1,22	27,000	Both & so	cs .	. OC41T
B-10 2.63	1	.— . Э	-	910./e10.	2.25	0.43/0.45	1,130	30,.00	Botn & 10	K	.coleB

SKIN: .OLU 'YO'O TE ALUMINUM CORE: .JOO IA JOJ2 LIUMINUM \* Retest CS - Core Jear FW - Pace 'Pilicia'r

TEST RESULTS - PANELS C & D Table .

_	TYPE Ao	FW .0003	9000*		1100. WH	FW 0006	1	9000	£0000° 1	1	cs .0010	cs .0007		6000° so	•	•	cs .0012	90000	1
	1	) <b>X</b> -1	<u>'</u>	'	pr.	P4.	'	E	, 	<u>'</u>	ى د		'	0	<u>'</u>	, 		'	. I
FAIL	LOCATION	Bot Ao	End	End	Bot Ao	Top Ao	End	Bot Ao	End	End	Bot & Ao	Bot @ Ao	End	Bot @ Ao	End	End	Bot & Ao	End	End.
	PSI	20,000	22,700	57,300	rded	21,200	64,700	46,600	700 و15	61,300	48,100	26,700	60,800	, 00°°9†	48,000	61,800	31,200	00%,46	2007
LOAD IN	POUNDS	2520	2970	3250	Not Recorded	1190	3630	2620	2900	3450	2415	3130	3365	2575	2715	3405	1724	3070	3135
	1/2	1	ı	. ,	1	1	1	1	1	1	•	·			I 		ı	1	•
BOTTOM	1	0.45	0.45	,	24.0	1	1	دَرَ•0	0.42	ı	0.45	0.32	1	درد.0	Side	1	0.30	0.4	1
	AT	.003	.002	0	±00°.	0	0	.0035	ر2002	0	ć£00°.	.002	0	700.	2005	0	.003	.0022	c
	- Zz	; !	ı	Low Corner	1	\cc.0	1	1	ı		-	l	ı	1	ı	ı	ı	l 	,
TOP	$\lambda_1$	•	ı		•	•	,	1	•	1		1	1	l	1	,	1		
	AT	٥	0	700.	0	/010.	0	0	0	0	0	0	0	0	0	0	0	0	_
:	WIDTH	2.82	2,82	2.42	2.82	2.82	2.82	2,32	2,32	2.82	2.76	2.76	2.76	2.76	2.76	2.76	2.76	2.76	37.0
Jade		C-2	c-3	h-0	C-5	9-0	)-J	g-2	6-0	01-0	9-9	D-3	7 0	D-2	9	D-7	8-0	D-9	9

ALUMINUM  $(I_1 = .125)$ SKIN: .010 /07,2-TC .CORE: .200 IA .052 CS - Core Shear FW - Face Wrinkling

Table 4 TEST RESULTS - PANELS E & F

SPEC.	L	-	TOP		-	BOTTOM		LOAD IN	_	FA TLUE	38	
NO.	MIDIM	Er,	71	1/2	T <sub>H</sub>	71	1/2	POUNDS	PSI	LOCATION	TYPE	Ao
<b>R</b> -2								Not Tested	ਰ			
<b>R-3</b>	2.75	.018/.013	1.9	0.30/0.48	/600*	1	/cċ•0	2060	46,800	Top & 40	FW	.0027
	2.75	.020/	1	0.6.7	.0071	1	0.4/	2680	61,000	Both 3 10	¥	,0014
<b>M</b>	2.75	/cto•	1	/01.0	410./410.	1.4	0.45/0.35	216;	49,500	Both & Ao	FW	.0016
9	2.75	0		1	0	1	ı	207>	47,500	Both ends	M	
1-1	2.75	/500:	1	/01.0	.0131	ı	09.0	2240	51,200	Bot @ Ao	¥	00100
8	2.7>	/900°	ı	0.35/	0	0	ı	2470	56,300	Top & Ao	ΜÆ	.0007
<b>8-9</b>	2.75	/900.	•	0.40/	/500.	1	0.22/	2620	60,000	Bot @ Ao*	M.	2000.
<b>18-1</b> 0	2.75	/900.	ı	0.35/	.003	9.		2630	000,009	Top & Ao*	FW	.0009 Bot
10	2.77	.028/	1	0.35/	/820	,	/05.0	0266	000	* C & C C C C C C C C C C C C C C C C C		9606
7	284	/800	•	7 1	) [ [0		781.0	2 1 0				Cramped
ĵ.		À		/20.0	/110.	ı	/01.0	2230	27,200	Bot 3 Ao	Ž,	.0027 Bot
†	2.87	.003/.007			100./400.	1.7	0.35/0.35	2990	67,500	Top @ Ac	£	.0025 Top
<b>₽</b>	2.87	.003/	1	0.20/	.003/	1	0.20/	066†	111,000	Both Ao	Æ	- 2000
φ	- <del></del>			Not Tested	ed							• •• ••
<b>JP-7</b>	2.87	/200.	ı	0.30/	.003	•	/00.0	2630	29,500	Top 40	£	.0010 Top
8	2.87	/c#0.	1	0.28/	/090.	ı	1.50/	2770	62,500	Top Ao	£	.0071 Top
6-4	2.87	/900.	1	/51.0	.0025	0.2		3200	72,300	Top Ao	M.	*800°
<b>F-1</b> 0	2.87	.0015 #1de	4.0	•	0	1	1	4180	, 91,200	Poss.Void	FW	9000.
			(010t iii) a	0	0			:	:		!	

SKIN: .008 1I-7 PH (TH 1050) STEEL ( $I_1$  = .178) CORE: .500 IA 50.2 ALUMINUM

\* Retest

CS - Core Snear FW - Face Wrinkling

Table 4 TEST RESULTS - PANELS E & F

SPEC.		Ĺ	TOP			BOTTOM		LOAD IN	_	FAILURE	E .	
NO.	WIDTH	<del>Т</del> ,		1/2	A <sub>T</sub>	71	1/2	POUNDS	P3I	LOCATION	TYPE	Ao
<b>B</b> -2								Not Tested				
<b>18</b> -3	2.(5	.018/.013	1.9	0.30/0.48	/600*	1	/cc.0	2060	46,800	Top & no	FW	.0027
<b>↑</b> -₩	2.73	.020/	ı	0.6.7	1,00.	1	0.4/	2680	61,000	Both & 10	FW	, 0014
<b>H</b>	2.75	/<10.	ı	0.40/	410./410.	η·τ	0.45/0.35	216;	49,500	Both & Ao	ΡW	.0016
9	2.75	0	•	1	0	1	ı	2075	47,200	Both ends	ΡW	,
<b>Z2</b>	2.75	·900·	ı	/01.0	.0131	1	09.0	2240	51,200	Bot & Ao	PW	0100.
8-8	2.75	/900.	ı	0.35/	0	0	ı	2470	56,300	Top & Ao	ΡW	7000.
<b>6-8</b>	2.75	/900-	ı	0.40/	/500.	ı	0.22/	2650	60,000	Bot @ Ao*	FW	.0007
<b>R</b> -10	2.75	/900.	1	0.35/	.003	9.		2630	000,09	Top & Ao*	MA	.0009 Bot
٩	1	/ 800		7.70	/ 1000		, 9:		3	1		
<u>.</u>		/ozo.	'	755.0	,020.	1	05.0	55/0	21,000	Top & Ao*	E	0036
<b>F-</b> 3	2.87	.008/	ı	);c.0	/110.	ı	0.18/	2530	57,200	Bot & Ao	ΡW	.0027 Bot
1	2.87	.003/.007			1,00./4,00.	1.7	0.35/0.35	2990	67,500	Top & Ao	PW	.0025 Top
<b>F</b>	2.87	.003/	1	0.20/	.003/	1	0.20/	066†	000,111	Both Ao	FW	2000.
9-4			·	Not Tested	p							
7-4	2.87	/100.	1	0.30/	.003	1	/٥٠٠٥	2630	29,500	Top 40	æ	.0010 Top
8-8	2.87	/c#0.		0.28/	/090.	ı	1.50/	2770	62,500	Top Ao	M.	.0071 Top.
6-6	2.87	/800.	•	0.15/	.0025	0.2		3200	72,300	Top Ao	M.	.003∗
<b>F-1</b> 0	2.87	.0015 #1de	ħ*0	· ;	0	ı	1	4180	91,200	Poss.Void	**	9000.
				0					!			

SKIN: .006 1/-7 PH (TH 1050)STERL ( $I_1 = .178$ ) CORE: .500 IA 50.2 ALUMINUM

\* Retest

CS - Core Snear FW - Face Wrinkling

10

TEST RESULTS - PANELS G & H Table 2

SPEC.			TOP			BOTTOM		LOAD IN		FATTIRE	TRE	
N	WIDEH	AŢ	$\square$	1/2	ΨV	1	72	POUNDS	PSI	LOCATION	TYPE	Ao
<b>G-</b> 2	2.81	.002 Side	2.0	-	0	-	,	3455	76,600	End		
<b>6-</b> 3	2.81	0	,	,	.025 Side	2.0		2280	50,700	End		
<b>†-0</b>	2.81	٥	ı	ı	ı	1	٠ ١	3930	87,100	End		
	2.81	.001> Side	2.0		0	1	ı	3150	69,800	End		
9-		Not Tested										
1-0	2.81	.030 Side	5,0		0	1	ı	1915	42,800	End		
8-0	2.81	۰	ı	ı	.008	ċ.		4450	99,100	End		
6-0	2.81	o	1	ı	.001 Dents	.25 Dia		3562	79,500	End		
0-10	2.81	0	1	ł	1	1	ı	2535	96,600	End		
H-2	2.71	0	l		.0012	0.3		4265	98,200	End	*	8000
н-3	2.71	.00	0.4		.020	Corner		1100	25,200	End		
<b>†</b> − <b>H</b>	2.71	0	,		,025	Вишр		1290	29,900	End		
Н-5	2.71	¢100°	0.35		0	1		1665		Possible Bond	nd	
9~H	2.71	.012 Dents	(long)		010./700.	2.0	a 55/0.7	3270	75,000	Bot Ao	*cs	.0005
<i>Ž</i> -н	2.71	0	i		0	•		0707	93,400	End	*	
8 <b>-</b> #	2.71	910.	۲.	ı	.002	4.0	ı	3340	72,200	Top @ Ao	cs	
6-н	2.71	.035/.035	1.3	0.35/0.30	0	,		1250	29,000	Top @ Ao	*CS	.0043
H-10	2.71	.020 Low	0.oDia		0	-		4740	109,000	End	*	
	. Table	Manage Wind	2 2 2 2	***************************************	0							

NOTE: Only Panels H-8 and -9 are represented on Figure 8--these are the only two failure panels. SKIN: .008 17-7 RH (TH 1050)STEEL( $f_1$  = .149 COPE: .250 5052 ALUMINUM

• Retest

CS - Core Shear FW - Face Wrinkling

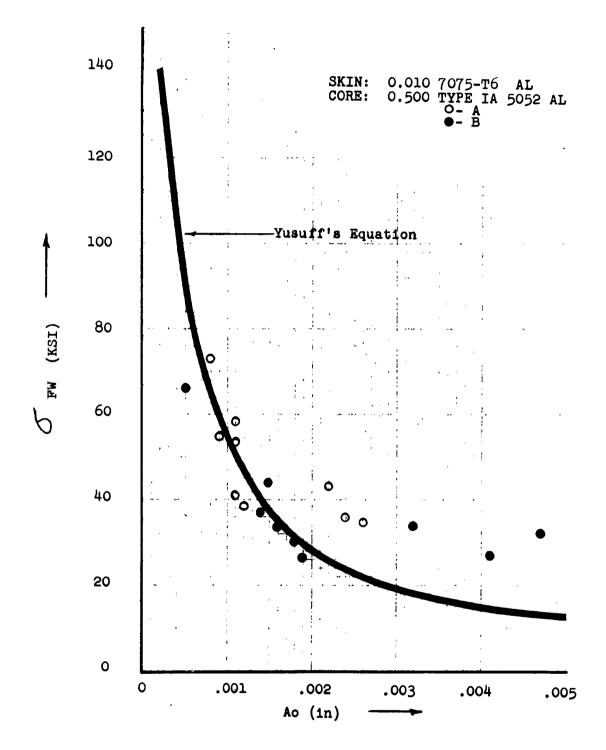


Figure 5 CRITICAL FACE-WRINKLING STRESS VS
SURFACE WAVINESS, A & B

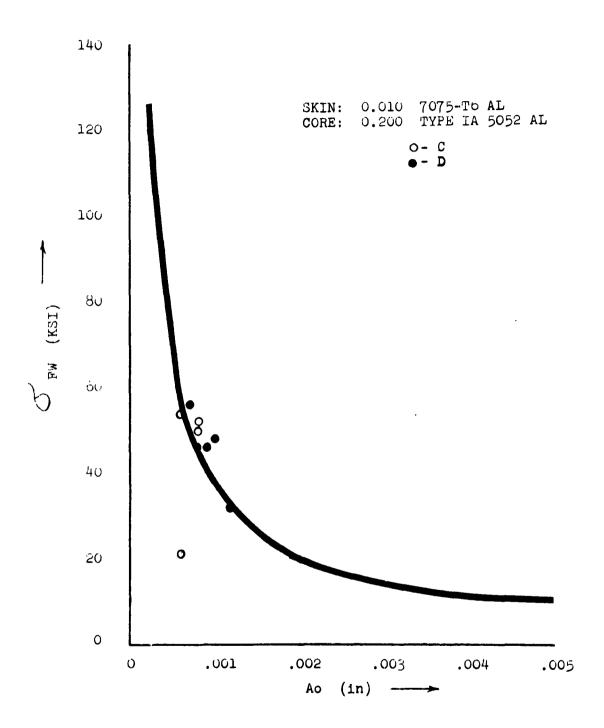


Figure 6 CRITICAL FACE-WRINALIAC ETHEES VS
SURFACE WAVINESS, C & D

-

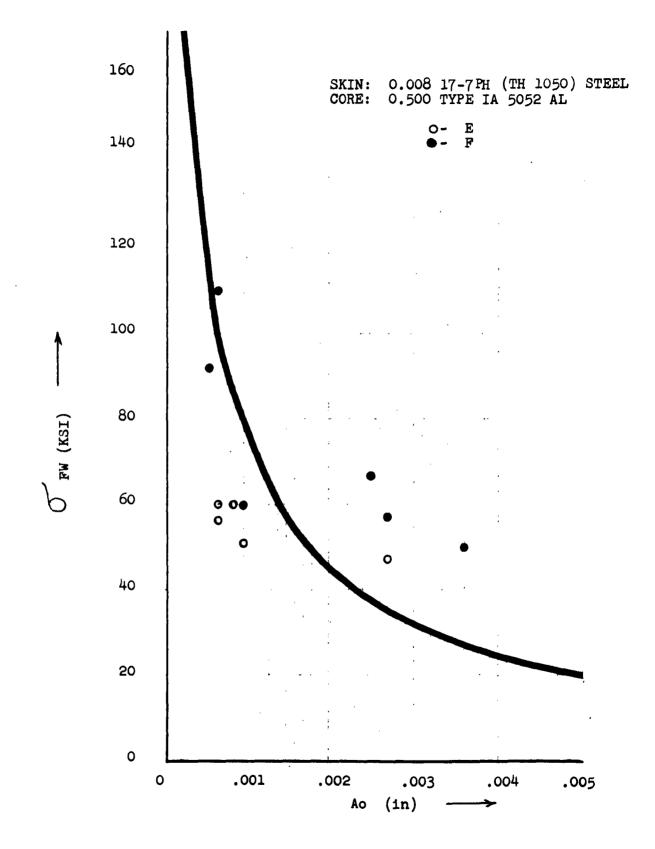


Figure 7 CRITICAL FACE-WRINKLING STRESS VS
SURFACE WAVINESS, E & F

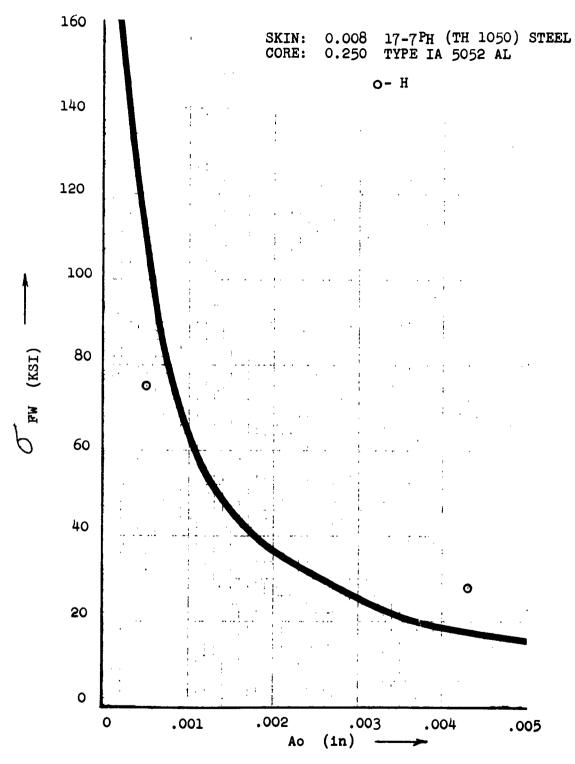


Figure 8 CRITICAL FACE-WRINKLING STRESS VS SURFACE WAVINESS, H

## TYPES OF FAILURE

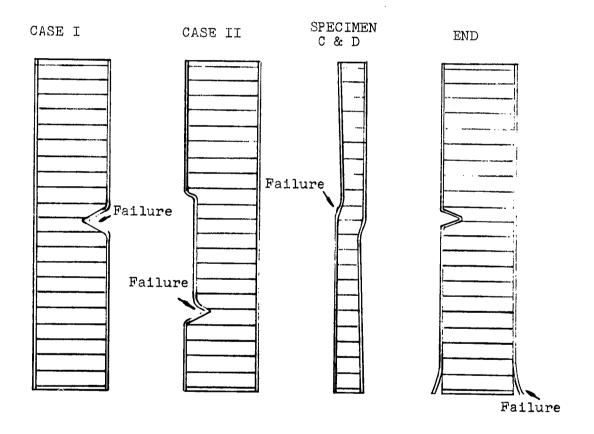


Figure 9

#### 3. CONCLUSIONS AND RECOMMENDATIONS

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The results of this experiment support the solution to the face wrinkling problem as proposed by Yusuff. Although the theory would be expected to apply only to small magnitude of waviness, it yielded reasonable correlation with extremely large degrees of waviness.

The writer does not feel that the mode of failure described by Equation (5) is critical. However, the typical failure of the type C and D (thin core) specimens (see Figure 9) indicates this could be the critical relationship. Calculations using the denominator of Equation (5) in Equation (3) show that the resulting critical stresses are roughly the same. Additional analytical work should be done in this area.

Application of this theory to a practical design situation requires an approach similar to that of thin shell construction. Either some arbitrary imperfection must be assumed or an empirical constant found. More extensive testing is required to establish the full validity of this theory. Should this be accomplished, a designer could use this relationship to establish the manufacturing tolerances on surface waviness necessary to ensure an adequate facewrinkling allowable.

#### 4. REFERENCES

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